

## 17 Attachment 2

Borough of Gettysburg

**APPENDIX A**  
**EXAMPLE VOLUME CONTROL STANDARD CALCULATIONS**

**Example 1**  
**Example Calculation of Runoff Capture Volume Requirement**

**Given:**

BMP CN: CN for individual land covers based on Table 2.2a (TR-55, SCS, 1986).  
 Hydrologic Soil Group = B

**Procedure:**

**Step 1:** For the pre-development condition, determine percentage of each land cover occurring on the site and the CN associated with each land cover.

Land Cover	HSG	CN	% of Site	Land Coverage (ft <sup>2</sup> )
Open space (good condition)	B	61	55	32,670
Woods (fair condition)	B	55	45	10,890

**Step 2:** Calculate the pre-development composite CN using a weighted average technique.

$$CN = \frac{61 * 32,670 + 55 * 10,890}{32,670 + 10,890}$$

$$CN = 59.5$$

**Step 3:** Calculate the required Runoff Capture Volume.

$$P = \text{Runoff Capture Volume} = \frac{200}{CN} - 2$$

$$P = \text{Runoff Capture Volume} = \frac{200}{59.5} - 2$$

$$P = \text{Runoff Capture Volume} = 1.36 \text{ inches}$$

For this hypothetical site, 1.36 inches of rainfall must be retained/infiltrated on the site.

# STORMWATER MANAGEMENT

## Example 2 BMP CN Calculation

### Given:

BMP CN: CN for individual land covers based on Table 2.2a (TR-55, SCS, 1986).  
Hydrologic Soil Group = B

### Procedure:

**Step 1:** Determine percentage of each land cover occurring on the site and the CN associated with each land cover.

Land Cover	HSG	CN	% of Site	Land Coverage (ft <sup>2</sup> )
Impervious (directly connected)	B	98	5	2,178
Impervious (unconnected)	B	98	10	4,356
Pervious pavement	B	70	5	2,000
Open space (good condition)	B	61	55	24,136
Woods (fair condition)	B	55	25	10,890

**Step 2:** Calculate the composite custom CN.

$$CN_c = \frac{CN_1 A_1 + CN_2 A_2 \dots + CN_i A_i}{A_1 + A_2 \dots + A_i}$$

$$CN_c = \frac{98 \times 2,178 + 98 \times 4,356 + 70 \times 2,000 + 61 \times 24,136 + 55 \times 10,890}{2,178 + 4,356 + 2,000 + 24,136 + 10,890}$$

$$CN_c = 65.4$$

**Step 3:** Calculate the BMP development CN based on the connectivity of site imperviousness.

$$CN_p = \frac{70 \times 2,000 + 61 \times 24,136 + 55 \times 10,890}{2,000 + 24,136 + 10,890}$$

$$CN_p = 59.7$$

$$R = \frac{10}{15}$$

$$R = 0.67$$

$$CN_c = CN_p + \left( \frac{P_{imp}}{100} \right) \times (98 - CN_p) \times (1 - 0.5R)$$

$$CN_c = 59.7 + \left( \frac{15}{100} \right) \times (98 - 59.7) \times (1 - 0.5 \times 0.67)$$

$$CN_c = 63.5$$

**Example 3**  
**Runoff Volume Calculation**

**Given:** Runoff Control Volume Requirement (Example 1) = 1.38 inches.  
Post development composite runoff curve number = 63.5 (Example 2)  
Development area = 43,560 square feet (Example 3)

$S = \frac{1000}{CN} - 10$	$Q(\text{inches}) = \frac{(P - 0.2S)^2}{(P + 0.8S)}$
$S = \frac{1000}{63.5} - 10$	$Q(\text{inches}) = \frac{(1.36 - 0.2 \times 5.7)^2}{(1.36 + 0.8 \times 5.7)}$
$S = 5.7$	$Q(\text{inches}) = 0.01 \text{ inches}$

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$$Q(\text{cubic feet}) = Q(\text{inches}) \times \frac{1 \text{ foot}}{12 \text{ inches}} \times \text{Area}$$

$$Q(\text{cubic feet}) = 0.01 \times \frac{1 \text{ foot}}{12 \text{ inches}} \times 43,560 \text{ square feet}$$

$$Q(\text{cubic feet}) = 36 \text{ cubic feet}$$

P = Required Runoff Control Precipitation Volume  
For this hypothetical case, P = 1.36 inches (Example 1)

CN = post-development runoff curve number  
For this hypothetical case, CN = 63.5 (Example 2)

Area = Development site area (square feet)  
For this hypothetical case, Area = 43, 560 (Example 2)

Q = "excess" runoff volume to be controlled by supplementary runoff volume control BMPs

Therefore: Approximately 36 cubic feet of runoff volume must be controlled using supplemental structural runoff volume control BMPs.

